

CITY OF SAN ÁNGELO PURCHASING DIVISION 72 West College Avenue, San Angelo, Texas 76903 Tel: (325) 657-4219

## ADDENDUM 3

Date: January 3, 2017

## RFB No: PK-02-17 / Construction of Concession Building

Bid Date: January 10, 2017/2:00 PM Local Time

The following clarifications, changes, additions, and/or deletions are hereby made to the RFB:

- What type of turf grass is currently used at the complex? Tifway 419 Hybrid Bermuda Grass.
- Has a design been created for the sign outside with the contractor, architect, etc. information? No, a sign is not required to be posted for this project, so no design has been specified.
- Refer to attached drawings A/2.01-D and S3.01-A Section E: Steel Joist & Metal Deck for metal deck specification.
- Change drawing B/A2.01-C Note: STAINLESS STEEL CABLE/FABRIC SYSTEM. RE: E/A2.02 reference to 8/A2.01-B.
- Alternate 1/Shade Fabric & Cable clarification: Refer to B/A2.01-A and note main radial cables extending to perimeter from center pole. These cables will be base bid. Alternate 1 will be the intermediate cables and fabric between the base bid cables described previously. The intermediate cables forming the two rings that will hold the fabric will be clamped to the main cables with fittings by Feeney or Equal. These fittings will be quick connect, swageless, with fixed end or tension adjustment fittings as required. These intermediate cables will provide just enough tension between main radial cables so that intermediate cables do not sag. This direction should be approved and coordinated with the fabric installer. Provide shop drawings for architect's approval before purchase and installation.

It is the responsibility of the vendor to ensure all addenda are considered in their pricing prior to submitting a bid by contacting the Purchasing Division at <u>SAPurch@cosatx.us</u>. Failure to do so will be at the risk of the vendors. No revisions to a bid can be allowed after the bids are opened for any reason.

Sincerely,

Candice Blake Purchasing Specialist



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Α.	GENERAL 1. do not scale drawings for the purpose of establishing dimensions.	BOND BEAM @ HEIGHT SPECIFIED REINF (2) #5 (TYP)	
	2. VERIFY EXISTING CONDITIONS AND DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO BEGINNING WORK.	CONT HORIZ REINF THRU CONTROL JOINTS	
	<ol> <li>REFERENCE ARCHITECTURAL, STRUCTURAL, AND MECHANICAL PLANS FOR VERIFICATION OF ALL BOLTS, BLOCKING, ANCHORS, ETC., AND THE ANCHORAGE OF THEIR RESPECTIVE ITEMS.</li> <li>COORDINATE WITH THE ARCHITECTURAL STRUCTURAL MECHANICAL PLUMBING AND ELECTRICAL DRAWINGS.</li> </ol>	END OF WALL	
	AND VERIFY THE LOCATION OF ALL CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, WALL OPENINGS, AND OTHER PROJECT REQUIREMENTS.		
•	DESIGN CRITERIA AND BUILDING CODE 1. INTERNATIONAL BUILDING CODE, 2009, INCLUDING AMENDMENTS AND ADDITIONS.		
	2. ROOF LIVE LOAD 20 PSF (REDUCIBLE)		BEARING SPAN
	3. ROOF DEAD LOAD 15 PSF		
	4. SNOW LOADS GROUND SNOW LOAD 5 PSF ROOF SNOW LOADS 5 PSF		
	SNOW EXPOSURE FACTOR0.9SNOW LOAD IMPORTANCE1.0		
	5. WIND LOADS BASIC WIND SPEED (ASCE –10) 115 MPH VELOCITY PRESSURE 24.5 PSF		
	lw 1.0 EXPOSURE C		
C.	STRUCTURAL STEEL		
	FY, KSI ASIM STRUCTURAL STEEL SHAPES, W AND WT 50 A992 STRUCTURAL STEEL SHAPES, OTHER THAN W AND WT 36 A36		
	STRUCTURAL STEEL PLATES 36 A36 HOLLOW STRUCTURAL SECTION (HSS), RECTANGULAR 46 A500, GR B		
	HOLLOW STRUCTURAL SECTION (HSS),       ROUND     42       HOLLOW STRUCTURAL SECTION (HSS),     50       A570       STRUCTURAL BOLTS     92	★	
	WELDING ELECTRODES     E70XX     AWS D1.1     00       1.     FABRICATION AND FRECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE		
	AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, CURRENT EDITION.		DOOR
	2. ALL STRUCTURAL STEEL MEMBERS AND CONNECTIONS SHALL BE DESIGNED USING AISC MANUAL OF STEEL CONSTRUCTION, LOAD AND RESISTANCE FACTOR DESIGN, SECOND EDITION.		
	<ol> <li>PROVIDE ALL WELDING DONE BY QUALIFIED, CERTIFIED WELDERS IN ACCORDANCE WITH AWS STRUCTURAL WELDING CODE-STEEL, D1.1:2000.</li> </ol>		JOINT FILLER
	4. STRUCTURAL DETAILS AND CONNECTIONS SHALL CONFORM TO THE STANDARDS OF THE AISC.		
	MINIMUM SIZE OF FILLET WELDS[b] Material Thickness of Thicker Minimum Size of Fillet		
	To 1/4 inclusive 1/8 Over 1/4 to 1/2 3/16		REF WALL REINF SCHED
	Over 1/2 to 3/4 1/4 Over 3/4 5/16		
	[a] Leg dimension of fillet welds. Single pass welds must be used. [b] See AWS Section J2.2b for maximum size of fillet		
	5. STRUCTURAL STEEL CONNECTIONS NOT DETAILED SHALL BE DESIGNED FOR 50%		VERT BAR POSITIONERS
(	INDICATED IN PART 2 (BEAMS AND GIRDERS) OF THE AISC MANUAL. MEMBERS REQUIRING CONNECTIONS OF GREATER CAPACITY THAN STATED ABOVE HAVE MEMBER REACTIONS SHOWN ON THE PLANS.	GROUT SOLID AT / ALL VERT REINF	W ISZDU VERTICAL
	6. THE FABRICATOR SHALL SUPPLY BACK-UP PLATES AND EXTENSION TABS FOR ALL COMPLETE PENETRATION WELDS.	REINFORCEMENT	Г
	<ol> <li>PROVIDE BEARING TYPE BOLTS AND INSTALL USING "TURN OF THE NUT" METHOD OR WITH TENSION INDICATING WASHERS.</li> <li>UNLESS NOTED OTHERWISE ALL LINEOLIAL LEG DOUBLE ANGLES SHALL HAVE</li> </ol>	$3 \frac{\text{DETAIL}}{NO}$	
	<ol> <li>ONELSS NOTED OTHERWISE, ALL ONEQONE LEG DOUBLE ANGLES SHALL HAVE LONG LEGS BACK TO BACK.</li> <li>CLEAN RUST, LOOSE MILL SCALE, AND OTHER FOREIGN MATERIALS FROM STEEL</li> </ol>		••••
<ul> <li>WHERE REQUIRED FOR FABRICATION, FITTING UP, OR WELDING.</li> <li>10. NO CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES IS ALLOWED WITHOUT PRIOR REVIEW AND WRITTEN APPROVAL OF THE ENGINEER.</li> <li>11. GROUT FOR BASE PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 6,000 PSI.</li> <li>STEEL CONNECTIONS TESTING</li> </ul>		<ul> <li>E. STEEL JOIST &amp; METAL DECK</li> <li>1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO STEEL DECK INSTITUTE</li> <li>INSTITUTES CODE OF RECOMMENDED STANDARD PRACTICE AND RASIC DESIGN SPECIFIC</li> </ul>	
		EDITION.	EB STEEL JOISTS K-SERIES" AS MANUFACTU
		<ul> <li>OR APPROVED EQUAL.</li> <li>3. STEEL FORM DECKING SHALL BE GALVANIZED METAL DECK OF THE HEIGHT AND WITH MINIMUM SECTION PROPERTIES AS FOLLOWS:</li> </ul>	
	<ol> <li>ARRANGE FOR TESTING AGENCY TO PERFORM ALL SHOP AND FIELD INSPECTION AND TESTING. SEQUENCE ALL WORK TO ALLOW TESTING REQUIREMENTS TO BE COMPLETED.</li> </ol>	ROOF - TYPE B H = 1-1/2 in Ga = 22 I = 0.169 in^4/ft Sp = 0.186 in^3/ft Sn = 0.192 in^3/ft Fy = 33 KSI 4. IMMEDIATELY AFTER ALIGNMENT, CONNECT DECKING TO SUPPORTS, INCLUDING PERIMETER MEMBERS PARALLEL TO SHEFTS WITH 5/8" DIAMETER PLIF	
	2. PROVIDE INSPECTIONS IN ACCORDANCE WITH AWS D1.1 ON ALL FABRICATED PIECES PRIOR TO SHIPPING AND ON ALL FIELD WORK ON THE SITE.		
	<ol> <li>REVIEW WELDERS CERTIFICATES.</li> <li>PERFORM MAGNETIC PARTICLE TESTING IN ACCORDANCE WITH ASTM E109 ON FILLET</li> </ol>		
WELDS AS FOLLOWS: 10% OF SHEAR PLATE FILLET WELDS AT RANDOM (FINAL PASS ONLY		AT 18" ON CENTER. PROVIDE SIDE LAP CONNECTION WITH #10 TEK SCREWS AT 18" ON CENTER. PROVIDE A MIN. OF 36 WELDS PER 100 SQ. F	
	100% OF TENSION MEMBER CONNECTIONS (I.E., HANGER RODS, X-BRACING) 10% OF OTHER MISCELLANEOUS WELDS.	6. ALL WELDING TO METAL DECK WILL CODE-SHEET STEEL, AWSD1.3-98.	PAN LENGTHS WHENEVER POSSIBLE. NO SIT
	VISUALLY INSPECT ALL REMAINING FILLET WELDS. 5. PERFORM ULTRASONIC TESTING ON ALL FULL PENETRATION WELDS.	7. SUSPENDED CEILING, LIGHT FIXTURE SUPPORTED BY THE STEEL DECK.	S, DUCTS OR OTHER UTILITIES SHALL NOT B
	6. PERFORM INSPECTION OF BOLTED CONNECTIONS IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS." TEST 10% OF ALL BOLTS IN BEARING TYPE CONNECTIONS (MINIMUM 1 BOLT PER CONNECTION)	F. CONCRETE MASONRY UNITS	
	TEST ALL FRICTION TYPE BOLTS. 7. VISUALLY INSPECT ALL IN PLACE DECK AND DECK CONNECTIONS. VERIFY DECK TYPE,	I. SIRUCIURAL CONCRETE MASONRY T STRUCTURES ACI530-99/ASCE 5-99/TM ACI530.1-99/ASCE 6-99/TMS 602-99, CONSTRUCTION	D DE IN ACCORDANCE TO BUILDING CODE R IS 402-99, SPECIFICATION FOR MASONRY S AND AC1531.1-76, 1983 SPECIFICATIONS F
5	GAGE, AND FINISH. REVIEW PLUG WELD CONNECTION QUALIFICATION PROCEDURES.	2. HOLLOW CONCRETE MASONRY UNITS REQUIREMENTS OF ASTM C90. CONCRE	TO BE LIGHT OR NORMAL WEIGHT, GRADE TE MASONRY UNITS TO BE MOISTURE CONTF
	REQUIREMENTS.	MINIMUM NET-AREA COMPRESSIVE STREI 3. PROVIDE MASONRY MORTAR MEETING 28-DAY MORTAR COMPRESSIVE STRENGT	исти ог 1,500 PSI. G THE REQUIREMENTS OF ASTM C270 OR AS H SHALL BE 1,800 PSI.
	STEEL JOIST & METAL DECK 1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO STEEL DECK INSTITUTES AND STEEL JOIST INSTITUTES CODE OF RECOMMENDED STANDARD PRACTICE AND PASIC DESIGN SUFCILIATIONS LATEST	4. PROVIDE MASONRY GROUT WITH MIN ACCORDANCE WITH ASTM C1019. GROU	IIMUM 28-DAY COMPRESSIVE STRENGTH OF T TO MEET THE MINIMUM REQUIREMENTS SF
	2. STEEL JOIST SHALL BE AN "OPEN WEB STEEL JOISTS K-SERIES" AS MANUFACTURED BY VULCRAFT	5. PROVIDE HORIZONTAL JOINT REINFO AND IN WALLS BELOW GRADE AT 8" ON I ADDER TYPE WITH CROSS TIES AT 16"	RCING IN WALLS ABOVE GRADE AT 16" ON ( CENTER VERTICAL SPACING JOINT REINFORC ON CENTER. GAI VANIZE WITH OR OUNCE 70
	OR APPROVED EQUAL. 3. STEEL FORM DECKING SHALL BE GALVANIZED METAL DECK OF THE HEIGHT AND GAGE SHOWN WITH MINIMUM SECTION PROPERTIES AS FOLLOWS:	6. LAY HOLLOW UNITS WITH FULL MOR WEBS, WHERE THEY ARE ADJACENT TO	TAR COVERAGE ON HORIZONTAL AND VERTIC
	ROOF - TYPE B H = $1-1/2$ in Gg = 22 (MAIN BLDG)	CONTAIN REINFORCING STEEL SHALL BE BOND BEAMS, LINTELS, AND PILASTERS. 7. VERTICAL CELLS TO BE FILLED WITH	FILLED SOLIDLY WITH 2,500 PSI CONCRETE
	I = 0.169  in  4/ft Sp = 0.186 in $3/\text{ft}$ Sn = 0.192 in $3/\text{ft}$	OPENING OF THE DIMENSIONS SHOWN C METHODS TO ENSURE COMPLETE FILLING	N DRAWINGS. GROUT TO BE CONSOLIDATED OF CELLS.
	4. IMMEDIATELY AFTER ALIGNMENT, CONNECT DECKING TO SUPPORTS, INCLUDING PERIMETER MEMBERS PARALLEL TO SHEFTS WITH 5/8" DIAMETER PUDDLE WELDS	9. INSTALL ANCHORS, ACCESSORIES, A	ND OTHER ITEMS TO BE BUILT IN AS WORK
	AT 18" ON CENTER. PROVIDE SIDE LAP CONNECTION WITH #10 TEK SCREWS AT 18" ON CENTER. PROVIDE A MIN. OF 36 WELDS PER 100 SQ. FT.	10. SPLICE VERTICAL REINFORCING A M	INIMUM OF 48 BAR DIAMETERS OR 24", WH TO BE KWIK BOLT, BY HILTI, INC., OR WEJ- RDANCE WITH THE CURRENT 1000 DEPOSIT
	6. ALL WELDING TO METAL DECK WILL CONFORM TO THE AWS STRUCTURAL WELDING CODE-SHEET STEEL, AWSD1.3-98.	12. FIELD DRILLED MASONRY ANCHORS	TO BE HIT HY20 INJECTION ADHESIVE ANCH
	7. SUSPENDED CEILING, LIGHT FIXTURES, DUCTS OR OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE STEEL DECK.	OR APPROVED EQUAL. INSTALL ANCHOF AND THE RECOMMENDATIONS OF THE MA	NS IN ACCORDANCE WITH THE CURRENT ICBO ANUFACTURER. DNRY SHALL BE DONF BY MASONRY MECHAN
		14. TOLERANCES FOR PLUMBNESS OF 20 FEET, AND $\pm 1/2$ " MAXIMUM. THEY	MASONRY WALLS OR COLUMNS SHALL BE ± SHALL ALSO MAINTAIN TRUE TO LINE WITHIN
		VARIATIONS FROM PLUMB. EACH COARS NO MORE THAN $\pm 1/2$ ".	SE SHALL REMAIN LEVEL WITHIN A SLOPE OF
		SHALL BE $3/8^{\circ} \pm 1/8^{\circ}$ . HEAD JOINT THICKNESS SHALL BE $3/8^{\circ} \pm 1/8^{\circ}$ . HEAD JOINT T	L DE DETWEEN 174 MIN. AND 374" MAXIM HICKNESS SHALL BE 378" (-174" TO +378 REQUIRED DURING THE CONSTRUCTION PROC
		G INSPECTIONS	
		1. CONTRACTOR SHALL CONTACT SKG EN TO INSTALLING INTERIOR OR EXTERIOR S	NGINEERING TO PERFORM A STRUCTURAL STE STUD FRAMING.
		2. PER SPECIFICATIONS, CONTRACTOR SI FOR APPROVAL PRIOR TO FABRICATION,	HALL SUBMIT STRUCTURAL STEEL SHOP DRAV REF. SPECIFICATIONS SECTION 05120.
		2. CONTRACTOR IS REQUIRED TO COORE	NATE WITH ALL OTHER ENTITIES THAT MAY



L STEEL INSPECTION, PRIOR

DRAWINGS TO THE ENGINEER MAY REQUIRE INSPECTIONS





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